

FAQs

1. Design a RCC circular cover slab of 7.5m clear diameter supported on 230mm thick beams which can provide complete fixity of the slab. The slab is used to support a parking load of 7.5 kN/m². Consider floor finish of 1.0 kN/m². Use M20 mix and Fe415 grade steel.

Diameter of slab = 7.5m clear and 7.73m effective

a. Load calculations:

i. Dead loads

$$\begin{aligned}\text{Assume thickness of slab from } l_x/d &= 20; \\ d &= l_x/20 \\ &= 386.5\text{mm} \\ D &= 386.5 + 15 + 10/2 \\ &= 406.5\text{mm}\end{aligned}$$

$$\text{Assume } D = 300\text{mm}; \quad d = 280\text{mm}$$

$$\text{Self-weight of slab} = 0.30 \times 25 = 7.50 \text{ kN/m}^2$$

$$\text{Weight of floor finish} = 1.00 \text{ kN/m}^2$$

$$\text{Total dead loads } w_d = 8.50 \text{ kN/m}^2$$

$$\text{ii. Live load on slab } w_l = 7.50 \text{ kN/m}^2$$

$$\text{iii. Total load on slab } w = 16.00 \text{ kN/m}^2$$

$$\begin{aligned}\text{b. Effective span } l_x &= 7.5 + 0.23 = 7.73\text{m} \\ &= 7.5 + 0.28 = 7.78\text{m whichever is less}\end{aligned}$$

$$\text{Hence } l_x = 7.73\text{m}$$

c. Bending moment calculations

- Circumferential moment at the centre of slab $(M_\theta)_c = + 1/16 wa^2 = 14.94 \text{ kNm}$
- Circumferential moment at the edge of slab $(M_\theta)_e = 0$
- Radial moment at the centre of slab $(M_r)_c = + 1/16 wa^2 = 14.94 \text{ kNm}$
- Radial moment at edge $(M_r)_e = - 2/16 wa^2 = 29.88 \text{ kNm}$

d. Effective depth of slab

Consider 1m width of the slab and by equating M_{umax} to M_{ulim}

$$M_{\text{umax}} = 1.5 \times 29.88 = 44.82 \text{ kNm}$$

$$M_{\text{ulim}} = 0.138f_{ck}bd^2 = M_{\text{umax}}$$

$$d = 127.44\text{mm} < 280\text{mm}$$

Hence safe against flexure.

$$\text{Keep } D = 300\text{mm}; \quad d = 280\text{mm}$$

e. Area of reinforcement

a. Main Steel for radial and circumferential moment at centre

$$M_u = (M_\theta)_c = (M_r)_c = 0.87f_y A_{st}(d - 0.416x_u)$$

$$A_{st, reg} = 228.97 \text{ mm}^2$$

i. Check for Minimum steel as per IS 456:2000

$$A_{st} = 0.12\% \text{ cross sectional area}$$

$$= 336 \text{ mm}^2 > 228.97 \text{ mm}^2$$

$$\text{Hence } A_{st reg} = 336 \text{ mm}^2$$

Provide 8mm diameter bar

$$\text{Spacing} = 149.60 \text{ mm}$$

Provide 8mm diameter bar at 125mm c/c

$$A_{st pro} = 402.12 \text{ mm}^2$$

b. Main Steel for radial moment at edge at top

$$M_u = (M_r)_e = 0.87f_y A_{st}(d - 0.416x_u)$$

$$A_{st, reg} = 459.16 \text{ mm}^2$$

i. Check for Minimum steel as per IS 456:2000

$$A_{st} = 0.12\% \text{ cross sectional area}$$

$$= 336 \text{ mm}^2 < 459.16 \text{ mm}^2$$

$$\text{Hence } A_{st reg} = 459.16 \text{ mm}^2$$

Provide 10mm diameter bar

$$\text{Spacing} = 171.05 \text{ mm}$$

Provide 10mm diameter bar at 150mm c/c as radial reinforcement at the edge at top for a length of $[7.96/2 - 3.98/\sqrt{3}] = 1.75\text{m}$ from the face of support. Provide 8mm diameter bar at 175mm c/c (minimum) as circumferential reinforcement at the edge. The circumferential reinforcement is a anchorage reinforcement which are used to avoid slippage of radial reinforcement at the edge at top.

Check for deflection

As per cl.23.2.1 of IS 456:2000

$$l/d = 20 \times M.F$$

$$p_t = 0.46\%$$

$$f_s = 0.58 f_y A_{st reg} / A_{st pro} = 220.77 \text{ N/mm}^2$$

$$\text{As per Fig.4; } M.F = 1.8$$

Hence $d = 183.33 \text{ mm} < 280 \text{ mm}$; Hence safe against deflection.