

### FAQs

1. Design a reinforced concrete slab for a room of size 4m x 5m clear subjected to a live load of 2.5 kN/m<sup>2</sup>. Three edges discontinuous (one long edge continuous). The load due to floor finish is 1 kN/m<sup>2</sup> and partition is 2 kN/m<sup>2</sup>. Use M20 and Fe415 as materials.

#### Three edges discontinuous (one long edge continuous)

Size of slab = 4.23m x 5.23m effective

The ratio  $l_y/l_x = 5.23/4.23 = 1.24 < 2$ ; it is two way slab.

#### a. Load calculations:

##### i. Dead loads

Assume thickness of slab from cl. 23.2.1 of IS 456:2000

$$l/d = 20;$$

$$d = 211.50\text{mm}$$

$$D = 231.5\text{mm}$$

Assume  $D = 150\text{mm}$

$$\text{Self-weight of slab} = 0.150 \times 25 = 3.75 \text{ kN/m}^2$$

$$\text{Weight of floor finish} = 1.00 \text{ kN/m}^2$$

$$\text{Partitions} = 2.00 \text{ kN/m}^2$$

$$\text{Total dead loads} \quad w_d = 6.75 \text{ kN/m}^2$$

$$\text{ii. Live load on slab} \quad w_l = 2.5 \text{ kN/m}^2$$

$$\text{iii. Total load on slab} \quad w = 9.25 \text{ kN/m}^2$$

#### b. Effective span

$$l_{\text{eff}} = 4.00 + 0.13 = 4.13\text{m}$$

$$= \text{c/c distance between supports} = 4.23\text{m whichever is less.}$$

$$\text{Hence } l_{\text{eff}} = 4.13\text{m}$$

#### c. Bending moment calculations: one long edge discontinuous

$$M_x = \alpha_x w l_x^2$$

$$M_y = \alpha_y w l_y^2$$

From Table 26 of IS 456: 2000; bending moment coefficients are

$$l_y/l_x = 5.23/4.23 = 1.24;$$

##### Short span coefficients

$$\text{-ve BM at continuous edge} \quad \alpha_x = 0.073$$

$$\text{+ve BM @ mid span} \quad \alpha_x = 0.0548$$

##### Long span coefficients

$$\text{-ve BM at continuous edge} \quad \alpha_y = \text{-----}$$

$$\text{+ve BM @ mid span} \quad \alpha_y = 0.043$$

$$\text{Max } M_x = 0.073 \times 9.25 \times 4.13^2 = 11.52 \text{ kNm}$$

$$\text{Max } M_y = 0.043 \times 9.25 \times 4.13^2 = 6.78 \text{ kNm}$$

$$M_{ux} = 1.5 \times 11.52 = 17.28 \text{ kNm}$$

$$M_{uy} = 1.5 \times 6.78 = 10.17 \text{ kNm}$$

**d. Effective depth of slab**

Consider 1m width of the slab and by equating  $M_{umax}$  to  $M_{ulim}$

$$M_{ulim} = 0.138 f_{ck} b d^2 = M_{umax}$$

$$d = 79.13 \text{ mm} < 130 \text{ mm}$$

Hence safe against flexure

$$\text{Keep } D = 150 \text{ mm}; \quad d = 130 \text{ mm}$$

**e. Area of reinforcement (along Shorter direction)**

Main Steel:

$$M_{ux} = 0.87 f_y A_{st} (d - 0.416 x_u)$$

$$A_{st, \text{reg}} = 393.25 \text{ mm}^2$$

Check for Minimum steel as per IS 456:2000

$$A_{st} = 0.12\% \text{ cross sectional area}$$

$$= 216 \text{ mm}^2 < 393.25 \text{ mm}^2$$

$$\text{Hence } A_{st \text{ reg}} = 393.25 \text{ mm}^2$$

Provide 8mm diameter bar

$$\text{Spacing} = 127.82 \text{ mm}$$

Provide 8mm diameter bar at 110mm c/c

$$A_{st \text{ pro}} = 456.96 \text{ mm}^2$$

**f. Area of reinforcement (along Longer direction)**

Main Steel:

$$M_{uy} = 0.87 f_y A_{st} (d - 0.416 x_u)$$

$$A_{st, \text{reg}} = 224.72 \text{ mm}^2$$

Check for Minimum steel as per IS 456:2000

$$A_{st} = 0.12\% \text{ cross sectional area}$$

$$= 216 \text{ mm}^2 < 224.72 \text{ mm}^2$$

$$\text{Hence } A_{st \text{ reg}} = 224.72 \text{ mm}^2$$

Provide 8mm diameter bar

$$\text{Spacing} = 223.68 \text{ mm}$$

Provide 8mm diameter bar at 200mm c/c

**g. Check for deflection**

As per cl.23.2.1 of IS 456:2000

$$l/d = 20 \times M.F$$

$$p_t = 0.35\%$$

$$f_s = 207.14 \text{ N/mm}^2$$

$$M.F = 1.6$$

$$d = 129.06 \text{ mm} < 130 \text{ mm}$$

Hence safe against deflection