

FAQs

1. Analyse and design the reinforcement required for a two way slab simply supported on all the four sides with corners free to lift or the slabs do not having adequate provisions to prevent lifting of corners. The effective dimension of the room is 3.5m x 3.5m. It is supported on 230mm thick wall. Live load on slab is 3 kN/m². Consider other dead loads also. Use M20 and Fe415 as materials.

Size of slab = 3.5m x 3.5m effective

The ratio $l_y/l_x = 3.5/3.5 = 1 < 2$; it is two way slab.

a. Load calculations:

i. Dead loads

Assume thickness of slab from cl. 24.1 of IS 456:2000

$$l/D = 35 \times 0.8;$$

$$D = 125\text{mm}$$

Assume $D = 125\text{mm}$

$$\text{Self-weight of slab} = 0.125 \times 25 = 3.125 \text{ kN/m}^2$$

$$\text{Weight of floor finish} = 0.025 \times 24 = 0.60 \text{ kN/m}^2$$

$$\text{Total dead loads } w_d = 3.725 \text{ kN/m}^2$$

$$\text{ii. Live load on slab } w_l = 3.00 \text{ kN/m}^2$$

$$\text{iii. Total load on slab } w = 6.725 \text{ kN/m}^2$$

b. Bending moment calculations

$$M_x = \alpha_x w l_x^2$$

$$M_y = \alpha_y w l_y^2$$

As per Table 27 of IS 456: 2000; bending moment coefficients are

$$l_y/l_x = 3.5/3.5 = 1; \alpha_x = \alpha_y = 0.062$$

$$M_x = M_y = 0.062 \times 6.725 \times 3.5^2 = 5.11 \text{ kNm}$$

$$M_{ux} = M_{uy} = 1.5 \times 5.11 = 7.665 \text{ kNm}$$

c. Effective depth of slab

Consider 1m width of the slab and by equating M_u to M_{ulim}

$$M_{ulim} = 0.138 f_{ck} b d^2 = M_u$$

$$d = 52.70\text{mm} < 105\text{mm}$$

Hence safe against flexure.

$$\text{Keep } D = 125\text{mm}; \quad d = 105\text{mm}$$

d. Area of reinforcement (along both directions)

Main Steel:

$$M_u = 0.87f_y A_{st}(d - 0.416x_u)$$

$$A_{st, \text{reg}} = 210.87 \text{ mm}^2$$

Check for Minimum steel as per IS 456:2000

$$A_{st} = 0.12\% \text{ cross sectional area}$$

$$= 126 \text{ mm}^2 < 210.87 \text{ mm}^2$$

$$\text{Hence } A_{st \text{ reg}} = 210.87 \text{ mm}^2$$

Provide 8mm diameter bar

$$\text{Spacing} = 238.37 \text{ mm}$$

Provide 8mm diameter bar at 200mm c/c

$$A_{st \text{ pro}} = 284.10 \text{ mm}^2$$

Torsion Steel:

$$A_{st} = \frac{3}{4} \text{ of Main steel } A_{st} = \frac{3}{4} \times 210.87 = 128.13 \text{ mm}^2$$

Use 8mm dia bars;

$$\text{Spacing} = 390 \text{ mm}$$

Provide 8mm dia bars at 300mm c/c at each corners both top as well as bottom in two layers at a distance of $l/5$ from the face of support.

e. Check for deflection

As per cl.23.2.1 of IS 456:2000

$$l/D = 35 \times 0.8$$

$$D = 125 \text{ mm}$$

Hence safe against deflection.