

FAQs

1. Design a simply supported floor slab for a room 8m x 3.5m clear in size supported on 230mm thick wall, if the superimposed load is 3 kN/m². Consider other dead loads also. Use M20 and Fe415 as materials. Draw the section of slab showing reinforcement details also.

$$\begin{aligned}\text{Size of slab} &= 8\text{m} \times 3.5\text{m clear} \\ &= 8.23\text{m} \times 3.73\text{m effective}\end{aligned}$$

$$\text{The ratio } l_y/l_x = 8.23/3.73 = 2.21 > 2 \text{ it is one way slab.}$$

a. Load calculations:

i. Dead loads

$$\begin{aligned}\text{Assume thickness of slab from } l_x/d &= 20; \\ d &= l_x/20 \\ &= 186.50\text{mm} \\ D &= 186.5 + 15 + 8/2 \\ &= 205.5\text{mm}\end{aligned}$$

$$\text{Assume } D = 150\text{mm}$$

$$\text{Self-weight of slab} = 0.15 \times 25 = 3.75 \text{ kN/m}^2$$

$$\text{Weight of floor finish} = 0.025 \times 24 = 0.60 \text{ kN/m}^2$$

$$\text{Weight of partitions (as per IS875)} = 1.96 \text{ kN/m}^2$$

$$\text{Total dead loads } w_d = 6.31 \text{ kN/m}^2$$

ii. Live load on slab $w_l = 3.00 \text{ kN/m}^2$

iii. Total load on slab $w = 9.31 \text{ kN/m}^2$

b. Effective span $l_x = 3.5 + 0.23 = 3.73\text{m}$
 $= 3.5 + 0.15 = 3.65\text{m}$ whichever is less
Hence $l_x = 3.65\text{m}$

c. Bending moment calculations

$$M_u = 1.5 \times 9.31 \times 3.65^2/8 = 23.26 \text{ kNm}$$

d. Effective depth of slab

Consider 1m width of the slab and by equating M_u to M_{ulim}

$$M_{ulim} = 0.138f_{ck}bd^2 = M_u$$

$$d = 91.80\text{mm}$$

$$D = 91.80 + 15 + 8/2 = 110.80\text{mm} < 150\text{mm}$$

Hence safe against flexure.

$$\text{Keep } D = 150\text{mm}; \quad d = 131\text{mm}$$

e. Area of reinforcement

a. Main Steel:

$$M_u = 0.87f_y A_{st}(d - 0.416x_u)$$

$$A_{st, reg} = 553.60 \text{ mm}^2$$

b. Check for Minimum steel as per IS 456:2000

$$A_{st} = 0.12\% \text{ cross sectional area}$$

$$= 157.2 \text{ mm}^2 < 553.60 \text{ mm}^2$$

$$\text{Hence } A_{st reg} = 553.60 \text{ mm}^2$$

Provide 10mm diameter bar

$$\text{Spacing} = 141.87 \text{ mm}$$

Provide 8mm diameter bar at 125mm c/c

$$A_{st pro} = 628.32 \text{ mm}^2$$

c. Distribution Steel

$$A_{st} = 0.12\% \text{ cross sectional area}$$

$$= 157.2 \text{ mm}^2$$

Use 6mm dia MS bar

$$\text{Spacing} = 176 \text{ mm}$$

Provide 6mm dia MS bar at 150mm c/c

f. Check for deflection

As per cl.23.2.1 of IS 456:2000

$$l/d = 20 \times M.F$$

$$p_t = 0.46\%$$

$$f_s = 0.58 f_y A_{st reg} / A_{st pro} = 213.77 \text{ N/mm}^2$$

$$\text{As per Fig.4; } M.F = 1.5$$

Hence $d = 123.33 \text{ mm} < 131 \text{ mm}$; Hence safe against deflection.