

FAQ's

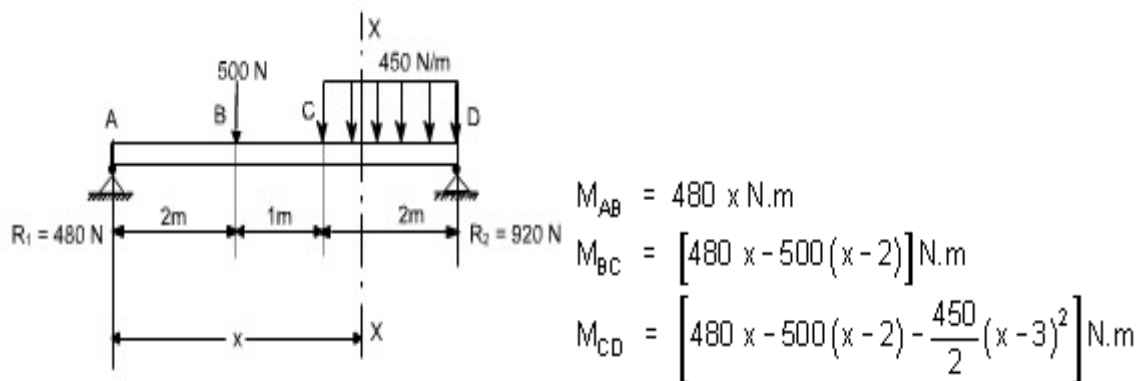
1. Explain the concept of Macaulay's method?

If the loading conditions change along the span of beam, there is corresponding change in moment equation. This requires that a separate moment equation be written between each change of load point and that two integration be made for each such moment equation. Evaluation of the constants introduced by each integration can become very involved. Fortunately, these complications can be avoided by writing single moment equation in such a way that it becomes continuous for entire length of the beam in spite of the discontinuity of loading.

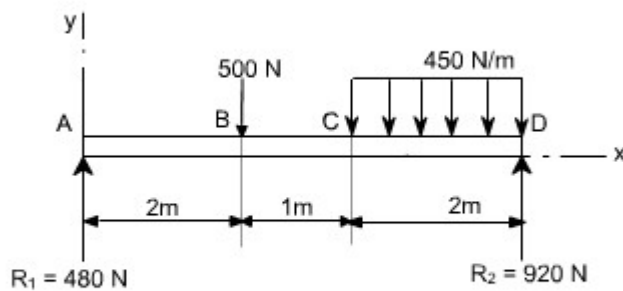
In Macaulay's method some author's take the help of unit function approximation (i.e. Laplace transform) in order to illustrate this method, however both are essentially the same.

For example consider the beam shown in fig below:

Let us write the general moment equation using the definition $M = (\sum M)_L$, Which means that we consider the effects of loads lying on the left of an exploratory section. The moment equations for the portions AB, BC and CD are written as follows



It may be observed that the equation for M_{CD} will also be valid for both M_{AB} and M_{BC} provided that the terms $(x-2)$ and $(x-3)^2$ are neglected for values of x less than 2m and 3m, respectively. In other words, the terms $(x-2)$ and $(x-3)^2$ are nonexistent for values of x for which the terms in parentheses are negative.

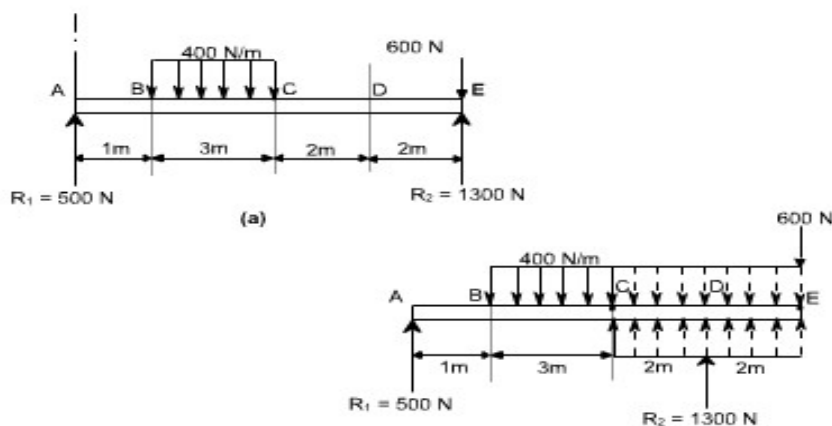


As an clear indication of these restrictions, one may use a nomenclature in which the usual form of parentheses is replaced by pointed brackets, namely, $\langle \rangle$. With this change in nomenclature, we obtain a single moment equation

$$M = \left(480x - 500 \langle x - 2 \rangle - \frac{450}{2} \langle x - 3 \rangle^2 \right) \text{N.m}$$

Which is valid for the entire beam if we postulate that the terms between the pointed brackets do not exists for negative values; otherwise the term is to be treated like any ordinary expression.

As an another example, consider the beam as shown in the fig below. Here the distributed load extends only over the segment BC. We can create continuity, however, by assuming that the distributed load extends beyond C and adding an equal upward-distributed load to cancel its effect beyond C, as shown in the adjacent fig below. The general moment equation, written for the last segment DE in the new nomenclature may be written as:



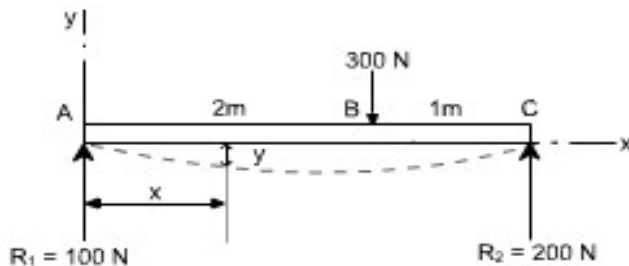
$$M = \left(500x - \frac{400}{2}(x-1)^2 + \frac{400}{2}(x-4)^2 + 1300(x-6) \right) \text{N.m}$$

It may be noted that in this equation effect of load 600 N won't appear since it is just at the last end of the beam so if we assume the exploratory just at section at just the point of application of 600 N than $x = 0$ or else we will here take the X - section beyond 600 N which is invalid. Procedure to solve problem is:

(i). After writing down the moment equation which is valid for all values of 'x' i.e. containing pointed brackets, integrate the moment equation like an ordinary equation.

(ii). While applying the B.C's keep in mind the necessary changes to be made regarding the pointed brackets.

2. A concentrated load of 300 N is applied to the simply supported beam as shown in Fig. Determine the equations of the elastic curve between each change of load point and the maximum deflection in the beam.



Writing the general moment equation for the last portion BC of the loaded beam,

$$EI \frac{d^2 y}{dx^2} = M = (100x - 300\langle x - 2 \rangle) \text{N.m} \quad \dots\dots(1)$$

Integrating twice the above equation to obtain slope and the deflection

$$EI \frac{dy}{dx} = \left(50x^2 - 150\langle x - 2 \rangle^2 + C_1 \right) \text{N.m}^2 \quad \dots\dots(2)$$

$$EI y = \left(\frac{50}{3}x^3 - 50\langle x - 2 \rangle^3 + C_1x + C_2 \right) \text{N.m}^3 \quad \dots\dots(3)$$

To evaluate the two constants of integration. Let us apply the following boundary conditions:

1. At point A where $x=0$, the value of deflection $y = 0$. Substituting these values in Eq. (3) we find $C_2 = 0$. keep in mind that $\langle x - 2 \rangle^3$ is to be neglected for negative values.
2. At the other support where $x = 3\text{m}$, the value of deflection y is also zero.

Substituting these values in the deflection Eq. (3), we obtain

$$0 = \left(\frac{50}{3} 3^3 - 50(3-2)^3 + 3.C_1 \right) \text{ or } C_1 = -133 \text{ N.m}^2$$

Having determined the constants of integration, let us make use of Eqs. (2) and (3) to rewrite the slope and deflection equations in the conventional form for the two portions.

segment AB ($0 \leq x \leq 2\text{m}$)

$$EI \frac{dy}{dx} = (50x^2 - 133) \text{ N.m}^2 \quad \dots\dots(4)$$

$$EI y = \left(\frac{50}{3} x^3 - 133x \right) \text{ N.m}^3 \quad \dots\dots(5)$$

segment BC ($2\text{m} \leq x \leq 3\text{m}$)

$$EI \frac{dy}{dx} = (50x^2 - 150(x-2)^2 - 133x) \text{ N.m}^2 \quad \dots\dots(6)$$

$$EI y = \left(\frac{50}{3} x^3 - 50(x-2)^3 - 133x \right) \text{ N.m}^3 \quad \dots\dots(7)$$

Continuing the solution, we assume that the maximum deflection will occur in the segment AB. Its location may be found by differentiating Eq. (5) with respect to x and setting the derivative to be equal to zero, or, what amounts to the same thing, setting the slope equation (4) equal to zero and solving for the point of zero slope.

We obtain

$50x^2 - 133 = 0$ or $x = 1.63$ m (It may be kept in mind that if the solution of the equation does not yield a value < 2 m then we have to try the other equations which are valid for segment BC)

Since this value of x is valid for segment AB, our assumption that the maximum deflection occurs in this region is correct. Hence, to determine the maximum deflection, we substitute $x = 1.63$ m in Eq (5), which yields

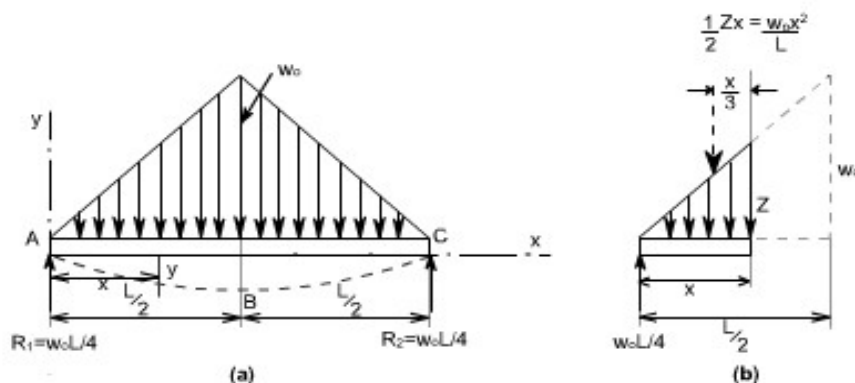
$$EIy|_{\max} = -145 \text{ N.m}^3 \quad \dots\dots (8)$$

The negative value obtained indicates that the deflection y is downward from the x axis. Quite usually only the magnitude of the deflection, without regard to sign, is desired; this is denoted by d , the use of y may be reserved to indicate a directed value of deflection.

If $E = 30$ Gpa and $I = 1.9 \times 10^6 \text{ mm}^4 = 1.9 \times 10^{-6} \text{ m}^4$, Eq. (h) becomes

$$\begin{aligned} y|_{\max} &= (30 \times 10^9) (1.9 \times 10^{-6}) \\ &= -2.54 \text{ mm} \end{aligned}$$

3. A simply supported beam carries the triangularly distributed load as shown in figure. Determine the deflection equation and the value of the maximum deflection.



Due to symmetry, the reactions is one half the total load of $1/2w_0L$, or $R_1 = R_2 = 1/4w_0L$. Due to the advantage of symmetry to the deflection curve from A to B is the mirror image of that from C to B. The condition of zero deflection at A and of zero slope at B do not require the use of a general moment equation. Only the moment equation for segment AB is needed, and this may be easily written with the aid of figure (b).

Taking into account the differential equation of the elastic curve for the segment AB and integrating twice, one can obtain

$$EI \frac{d^2 y}{dx^2} = M_{AB} = \frac{w_0 L}{4} x - \frac{w_0 x^2}{L} \cdot \frac{x}{3} \quad \dots\dots(1)$$

$$EI \frac{dy}{dx} = \frac{w_0 L x^2}{8} - \frac{w_0 x^4}{12L} + C_1 \quad \dots\dots(2)$$

$$EI y = \frac{w_0 L x^3}{24} - \frac{w_0 x^5}{60L} + C_1 x + C_2 \dots\dots(3)$$

In order to evaluate the constants of integration, let us apply the B.C's we note that at the support A, $y = 0$ at $x = 0$. Hence from equation (3), we get $C_2 = 0$. Also because of symmetry, the slope $dy/dx = 0$ at mid span where $x = L/2$. Substituting these conditions in equation (2) we get

$$0 = \frac{w_0 L}{8} \left(\frac{L}{2}\right)^2 - \frac{w_0}{12L} \left(\frac{L}{2}\right)^4 + C_1 C_1 = -\frac{5w_0 L^3}{192}$$

Hence the deflection equation from A to B (and also from C to B because of symmetry) becomes

$$EI y = \frac{w_0 L x^3}{24} - \frac{w_0 x^5}{60L} - \frac{5w_0 L^3 x}{192}$$

Which reduces to

$$EI y = -\frac{w_0 x}{960L} (25L^4 - 40L^2 x^2 + 16x^4)$$

The maximum deflection at midspan, where $x = L/2$ is then found to be

$$\boxed{EI y = -\frac{w_0 L^4}{120}}$$